

Engineers Report in Accordance with IS 465 :2018



Consulting Structural
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1.0 Introduction

IS:465:2018+A1:2020 Assessment, Testing and categorisation of damaged buildings incorporating concrete blocks containing certain deleterious materials.

This is the current standard on which the testing is based upon. The protocol of the standards recommends a building condition assessment by the Engineer to categorise the level of damage and group the property within groups 1-4. There was no damage attributable to defective blockwork evident.

This property is group 1, undamaged. The protocol in IS:465 states that no core extraction or coring should take place unless any damage to the property places the property in at least Group 2.(table 1) Group 1 properties are outside the scope of IS:465. If the property is undamaged, then no testing would be recommended.

The testing in this dwelling has been requested to prove the blockwork is not defective for a house sale. We have extracted 5no. cores from the dwelling, which I feel is sufficient to get an indication of the blockwork performance.

Suite A, Suite B (Petrographic and SEM testing) and sulphide analysis have been conducted.

2.0 Brief

On the instructions of our client [REDACTED], we have been asked to confirm the presence of defective blockwork at the above project.

3.0 Observations

In association with Anytime Coring, we have extracted cores as follows:

Core 1	LHS Gable Elevation (outer leaf)
Core 2	LHS Gable Elevation (inner leaf)
Core 3	Front Elevation (outer leaf)
Core 4	Front Elevation (inner leaf)
Core 5	Rear Elevation (substructure)

The dwelling is a 4 bedroom semi detached house.

Suite A simplified petrography analysis, Suite B examination and classification requirements, estimation of free muscovite mica and compressive strength tests.

4.0 Results

4.1 Suite A Analysis.

Suite A has identified one type of concrete.

Cores 1, 2, 3, & 5 have been classified by Suite A examination as displaying a LOW/MEDIUM susceptibility to deterioration due to the common (trace/minor) presence of potentially problematic mica bearing lithologies / minerals.

4.2 Compressive Testing

The compressive test results have indicated strengths of

Core 4, 13.40N/mm²

4.3 SEM Petrographic Analysis.

Core 1 was submitted (SEM) petrographic testing and sulphide analysis.

The aggregate in concrete type 1 is composed of a 0/25 mm river gravel of chloritised sandstone, chloritised siltstone and weathered sandstone.

SEM analysis of the binder of sample 1 (selected as representative of the concrete type) quantifies the average content of free muscovite mica at 2% by volume of cement paste. This is considered to be an underestimate as discrimination of free mica by SEM analysis is not absolute. There is also evidence of free chlorite, which is also a phyllosilicate and could behave similarly to muscovite in cement. While there are no published limits for free mica content of aggregates in European or American standards, it has been reported that in most cases of poor concrete block durability due to micaceous aggregates that the free mica content of the binder is typically >5% by volume of the paste. The free mica content is below this proposed threshold and associated with aggregate components that typically perform well during freeze-thaw testing.

The presence and abundance of free mica in the binder, as determined by the supplementary testing, confirms the Suite A classification of concrete type 1 as displaying Low/Medium susceptibility to potential free mica degradation.

(RISK LEVEL 2) Risk (of mica degradation) levels subjectively take into account both amount and context: 1 – Negligible; 2 – Low/Medium; 3 – High; 4 – Very High/Critical.

The aggregate contains only traces of pyrite (visual estimate < 0.25%), with common iron oxides likely formed from the oxidation of the sulphides. The sulphides are generally encapsulated in stable lithologies. The presence of sulphides can be a cause for concern due to the potential for oxidation and subsequent production of expansive cracking and associated secondary sulphate minerals. However, in this sample the pyrite is generally well encapsulated or already converted to iron oxides and only trace sulphides are visible.

The reported cement content was 14.2 & 12.3% (core 1 & 5)

4.4 Sulphide Chemical Analysis.

Results of a sulphide risk assessment are detailed below.

Test	Core 1
Total sulphur content (as S) by HTC	0.18%
Acid soluble sulphate content (as SO ₄)	0.30%
Water soluble sulphate content (as SO ₄)	<50mg/l
Equivalent residual pyrite equivalent	0.15%

The chemically determined residual sulphide mineral content (pyrite equivalent value) was calculated as 0.15% for sample 1. Acid soluble sulphate (AS) content was reported as 0.30% SO₄ and water-soluble sulphate (WSS) content was reported as <50 mg/l SO₄. The AS value is below the guidance value of 0.5% SO₄ mentioned in Annex E of IS 465:2018+A1:2020 as a value of sulphate that would not be considered abnormal in a standard concrete block.

(RISK LEVEL 1) Risk (of pyrite or other sulphide degradation) levels subjectively take into account both amount and context: 1 –Negligible; 2 – Low/Medium; 3 – High; 4 – Very High/Critical.

Total sulphur (TS, 0.18%) slightly exceeds the 0.1% value mentioned in Annex E as a typical contribution to total sulphur from the cement for a standard block. Irish Standard EN 12620:2002 and Irish national guidance S.R. 16:2016 limit total sulphur in aggregate for use in concrete to ≤1.0%TS. In sample 1, after sulphur in the cement has been accounted for, the TS remains significantly below this limit.

(RISK LEVEL 1) Risk (of pyrite or other sulphide degradation) levels subjectively take into account both amount and context: 1 –Negligible; 2 – Low/Medium; 3 – High; 4 – Very High/Critical.

The chemical results for sample 1, taken together with the textural encapsulation of the majority of the pyrite, means the risk of concrete degradation from sulphide oxidation is considered Negligible.

5.0 Conclusions

The concretes should be considered as displaying LOW/MEDIUM Risk to deterioration from potential freeze-thaw damage.

The concrete should be considered as displaying NEGLIGIBLE Risk to deterioration from sulphide degradation.

In my opinion the blockwork is suitable for construction if all blockwork is characteristic of this test sample.



Signed _____

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